

September 9, 2021

To:

Brian Grennell, Cultural Resources Management Coordinator State Historic Preservation Office 300 North Washington Square Lansing, MI 48913

From:

Robb Roos, Project Manager and Senior Environmental Analyst Mike Madson, Cultural Resources Specialist Angela Julin, Cultural Resources Specialist

Subject:

Four Lakes Task Force: Smallwood Dam Rehabilitation Smallwood Dam, Midland County, Michigan

Dear Mr. Grennell:

With this letter Four Lakes Task Force (FLTF) provides your office information on the efforts to stabilize the Smallwood Dam Midland County, Michigan. The Smallwood Dam is located on the Tittabawassee River, a tributary to the Saginaw River, and is approximately 35 river miles upstream of the City of Midland in Midland County, Michigan (see **Figures 1 and 2**). The facility is owned and operated by the FLTF which is the Part 307 delegated authority for Gladwin County. Construction of the dam was completed in 1925 to provide storage and headwater control for the purpose of hydroelectric power generation.

From left to right (looking downstream), the Project consists of an approximately 1,000-foot-long left embankment, an approximately 52-foot-wide gated spillway with two Tainter gates, an approximately 25-foot-wide powerhouse containing one turbine generating unit with a rated capacity of 1.2 MW with an operating head of 29.6 feet, and an approximately 125-foot-wide right embankment. The dam's normal headwater and tailwater elevations are approximately 704.3 and 674.7, respectively. All elevations presented are in the North American Vertical Datum of 1988 (NAVD88). The primary goal of the Smallwood Dam Rehabilitation Project is to construct improvements in accordance with the State of Michigan Department of Environment, Great Lakes and Energy (EGLE) requirements and restore the legal Smallwood Lake level.

Merjent, Inc. is a subcontractor to FLTF's consultant Spicer Group. In April 2021, Merjent archaeologists initiated an archaeological survey of the Smallwood Dam, and the results of that survey are pending. An architectural history review was not part of the initial scope; however, consideration of Smallwood Dam's structural components is now of interest to parties consulting with the FLTF, most notably the U.S. Department of Agriculture – Rural Development (USDA-RD). Recently, the USDA-RD advised FLTF to communicate with your office on the measures being taken to stabilize failed dams across the FLTF area of concern, including interim and long-term proposed rehabilitation measures at the Smallwood Dam. The proposed rehabilitation efforts for the Smallwood Dam are depicted on the Smallwood Dam Conceptual Design, which is

provided as **Appendix A**. These plans depict the proposed work that is discussed within the narrative sections of this report.

Smallwood Spillway

The reinforced concrete spillway is a hollow reinforced concrete barrel arch and ogee-shaped rollway structure spanning to buttress piers and powerhouse wall and left spillway training wall with two Tainter gate bays (see Photo 1). Both the left and right Tainter gates are 25.4-feet-wide by 10-feet-high separated by a 1.5-foot-wide center pier. The spillway ogee crest sill is at El. 694.3 feet. A hydraulic hoist operates the gates with the operators located directly adjacent to the hoist above each gate on an elevated platform. The hydraulic gate chain and single cable hoist and reel system was installed in 2019, replacing the original electric hoist and trolley system. The gates are currently in the fully open position (10-feet) and dogged off to maintain a drawndown impoundment per Federal Energy Regulatory Commission (FERC) and EGLE orders and to prevent accidental closure in the event of gate hoist or hoist cable failure.

Smallwood Dam Powerhouse

The powerhouse consists of a reinforced concrete substructure and brick superstructure with one vertical Francis shaft unit. A vertical trumpet-shaped draft tube is installed in the lower portion of the powerhouse allowing discharge of the outflow into the spillway stilling pool. The tailrace slab slopes up to the natural river bottom. Both spillway and powerhouse structures are constructed on dense glacial till. The base slabs for both contain shear keys and an upstream concrete cutoff into the till. The powerhouse measures 27 feet by 28 feet by 60 feet in height.



Photo 1. Smallwood Spillway and Powerhouse, October 2020.

Smallwood Dam Embankments

The left embankment is approximately 1,000-feet long, with a maximum structural height of 38 feet near the spillway. The embankment consists of an approximately 320-foot-long non-overflow section nearest the spillway and an approximately 680-foot-long emergency overflow section. The original embankment was reportedly constructed of native silt, sand, and clay from onsite sources. The non-overflow section contains a steel sheet pile (SSP) cutoff constructed in 1999 along the upstream edge of the embankment crest. The SSP was driven through the embankment and foundation overburden soils into the glacial till and extends above the crest to El. 715.2. In 2000, the upstream SSP line was extended downstream to form a training wall between the non-overflow and emergency overflow sections. The crest of the non-overflow section was narrowed, and the downstream slope regraded and flattened in 2001 to satisfy global stability criteria. The left embankment non-overflow section and SSP are illustrated Photo 2. The crest elevation of the 680-foot-long left embankment emergency overflow section generally ranges from El. 709.0 to 711.5 feet. This section is intended to overtop during an extreme flood event.



Photo 2. Smallwood Dam Left Embankment, December 2020.

The right embankment is 125-feet long, 38-feet tall and reportedly constructed of native silt, sand, and clay material. The right embankment also contains an upstream SSP seepage cutoff constructed in 1999 that extends approximately 250 feet beyond the end of the powerhouse abutment wall (see Photo 3).



Photo 3. Aerial Drone Imagery of Smallwood Dam, May 2020.

Smallwood Dam Stabilization

Following the May 19, 2020, storm event, significant downstream and embankment erosion damage occurred at the Smallwood Dam. In order to prevent similar dam failure events, most notably as seen at the Edenville Dam, the following rehabilitation measures are proposed:

- Partially demolish the existing Tainter gate spillway and replace with new hydraulically operated crest gates to increase spillway capacity.
- Raise the spillway training walls and extend the walls downstream to accommodate the increased outflows.
- Fill the hollow spillway rollways with mass concrete.
- Construct a new pin flashboard auxiliary spillway across the top of the left embankment to assist in passing the inflow design flood (IDF).
- Retrofit the existing powerhouse with a low-level outlet to pass base river flows during the winter. For Smallwood Dam, the current base river flow is a minimum of 80 cubic feet per second (cfs) in the winter months.
- Flatten the left and right embankment slopes to improve stability. The overflow section
 of the left embankment will be raised and extended east to "tie-in" to high ground at the
 left abutment.

SUMMARY OF DAM REPAIRS AND FLOOD CAPACITY UPGRADES

Primary Spillway Modifications

The existing Tainter gate spillway will be partially demolished and the two (2) Tainter gates will be replaced with hydraulically operated crest gates at approximate sill El. 688.3 to increase the spillway capacity. The left crest gate (Bay No. 2) and the right gate (Bay No. 1) will both be approximately 22.6-feet-wide by 16-feet-high. The new gates will be designed to open and close locally, using hydraulic controls (i.e., no remote or automatic operation capabilities) during both normal operation and during flood events. The hydraulic gate operators will be supported on a new, reinforced concrete center pier. The upstream portions of the barrel arches below approximate El. 688.3 will remain and the crest gates and their anchorage embedments will be founded on mass concrete over the original existing reinforced concrete slab. A reinforced concrete stepped chute will convey water that discharges over the crest gates down to a new reinforced concrete stilling basin. Both the left and right spillway walls and center pier will be widened, extended downstream, and side walls raised to provide adequate flow clearance, prevent overtopping, and accommodate downstream slope flattening of the flanking embankments.

Powerhouse Modifications

The FERC license was surrendered in May 2021 and flows through the powerhouse turbines ceased. The dam therefore has no means to pass base river flows other than over the spillway sill. For the long-term reconstruction, the existing powerhouse and its water passages will be retrofitted to pass base flows (estimated at 100 to 300 cfs) to reduce spilling over the crest gates during winter months and pass minimum monthly flows in accordance with the State of Michigan EGLE minimum flow requirements. This will also reduce the build-up of ice on the spillway walls due to spray kicked up from the rollway and stilling basin. This will be accomplished by removing the existing generator, turbine shaft, ancillary mechanical and electrical equipment, installing a

bulkhead over the runner pit and fixing the runner into place. The existing wicket gates and timber headgates will be repaired and upgraded. Upstream stop log slots and a new inclined trash rack will be installed to allow maintenance to the new intake and protect the gate and runner from flotsam and floating debris.

Auxiliary Spillway

A flashboard-type overflow spillway will be constructed across the top of the left embankment at approximate EI. 705.5 to pass flows in excess of what the primary spillway and low-level outlet can pass. Stainless-steel pipe stanchions (pins) will be set into pockets embedded in the concrete base slab and will support corrosion-resistant, plate flashboards that extend up to approximate EI. 709.5. The stainless-steel pin supports will be designed to fail during an extreme flood event that overtops the flashboards by controlled buckling failure to provide additional spillway capacity to pass flows up to the IDF. The overflow spillway will discharge into a concrete chute and U.S. Bureau of Reclamation (USBR) Type III stilling basin to dissipate energy and convey flow into the downstream discharge channel. Downstream of the stilling basin, the IDF design storm will be routed approximately 350 feet downstream to the confluence with the Tittabawassee River in a rock-lined spillway discharge channel.

EMBANKMENT MODIFICATIONS

The upstream and downstream embankment slopes will be flattened, and the crest widened to at least 15 feet to provide adequate stability in accordance with EGLE requirements under normal and flood pool loading criteria. The upstream slope will be graded to 2.5H:1V and new rip-rap over bedding will be placed to protect the dam during summer and winter pools and during flooding events. The overflow section of the left embankment will be raised to approximate El. 715.0 and extended approximately 700 feet to the east to "tie-in" to high ground at the approximate El. 715.0 topographic contour. The downstream slope will be flattened to 3H:1V to allow safe mowing and improve stability. The downstream overlay fill will include an internal filter and drainage layers will be installed to protect against seepage-induced internal erosion. The drainage systems will discharge to a weir to allow monitoring of seepage rates.

Embankment Fill

New embankment fill placed on maximum 2.5H:1V upstream slopes and 3H:1V downstream slopes will be used to reconstruct the embankment sections. The embankment fill will consist of material either salvaged from onsite excavations or imported from approved off-site sources, as required. All cobbles greater than 6 inches in diameter, or 2/3 the height of the allowable lift thickness, whichever is less, will be screened out. New embankment fill material will be comprised of semi-pervious granular material (Unified Soil Classification System [USCS] soil types: SP-SM, SM, and SC-SM) will be compatible with the remaining, existing embankment fill in terms of filter criteria. Embankment fill will be placed in loose horizontal lifts not exceeding 12 inches and compacted in a controlled manner to a minimum of 95 percent of maximum dry density determined by the standard Proctor (ASTM D698) with appropriate moisture control measures per ASTM D698.

Steel Sheet Pile

A sheet pile cutoff wall will be constructed in an approximately 125-foot-long section of the left embankment immediately to the left of the proposed auxiliary spillway along the upstream crest. The sheet pile is anticipated to be a hot rolled PZC 14 Grade 50 ksi or approved equal with WADIT

Interlock Sealant. No hydrophilic interlock sealants will be allowed as they are frost susceptible to break down and weathering.

Reverse Filter and Toe Drain

A reverse filter toe drain consisting of filter sand and drainage stone blanket will be constructed at the downstream slope and toe of the left and right embankments to mitigate against seepage and internal erosion of the embankment and foundation soils. The reverse filter and toe drain will generally consist of 18 inches of fine filter (MDOT 2NS natural riverbank or river bottom sand) and 24 inches of coarse filter (MDOT 29A stone). The MDOT 2NS shall not be a manufactured sand from carbonate bedrock. Depending upon their condition upon excavation, the existing finger drains will either be extended and conveyed downstream to daylight at the toe or be terminated with stainless-steel wire screened ends to pass flow, but no soil fines and seepage will be collected in the drainage stone layer. The seepage will be collected in a minimum 8-inch diameter slotted drainpipe surrounded by coarse filter material within an envelope of natural filter sand. The purposes are 1) to provide an outlet to convey seepage toward the outlet to keep the phreatic surface from rising within the reverse filter, and 2) to collect and direct seepage flow entering the reverse filter to the downstream weir box so the flow rate and potential fines movement can be collected and monitored.

Rip-rap and Bedding

Rip-rap placed on the upstream side of the auxiliary spillway approach apron, and upstream and downstream embankment slopes will consist of a hard, durable, non-weathered, angular stone in accordance with Michigan Department of Transportation (MDOT) standard specifications. The rip-rap design will follow Federal Emergency Management Agency's (FEMA) "Filters for Embankment Dams, Best Practices for Design and Construction," (FEMA, 2011) and will be sized in accordance with MDOT Standard Specifications for Construction. Bedding material will consist of imported granular material in accordance with MDOT specifications placed over MDOT 29A crushed stone or heavy non-woven geotextile (≥ 11 oz./sy). Geotextile will only be allowed under the upstream slope or under bedding located under the toe seepage collection ditch. Geotextile will not be permitted under the downstream slope.

SMALLWOOD DAM ARCHITECTURAL HISTORY

Frank Wixom founded the Tittabawassee Power Company in 1906 with a vision to dam the Tittabawassee and Tobacco River to provide cheap electricity to farmers and the local community, and to create a recreational area for the greater region. Although the initial plans were not financially feasible at the time for Wixom's power company, the Wolverine Power Company was formed in 1923, with Wixom as president. Sponsored by Wolverine Power Company, construction of the Smallwood Dam, designed by consulting engineers Holland, Ackerman & Holland, was completed in 1925 to provide storage and headwater control for the purpose of hydroelectric power generation.

FLTF maintains a collection of historic photos taken during the construction of the Smallwood Dam, as well as sheets of construction plans and improvements (1920s), as-builts (1930s), and common lifecycle modifications (into the 2000s). Original plans for the Smallwood Dam are provided as Figure 3.

Smallwood Dam as a Cultural Resource

To understand the eligibility of the Smallwood Dam as a cultural resource, all built components of the dam were considered. This includes the concrete spillways, water retaining, or water

conveyance structures described in this report including the primary gated spillway (comprised of side walls, center piers, stepped rollway, stilling basin and crest gates), powerhouse (side walls, intake, scroll case, draft bay), and auxiliary spillway (including side walls). None of these elements have previously been evaluated for inclusion on the National Register of Historic Places (NRHP).

Spillway construction began in September 1923 (see Photo 4), followed by the powerhouse. Once the spillway structure was built, embankment dams were built using the native Michigan sand-clay materials for the fill, founded on the hardpan glacial till of the region. The powerhouse structure was built first and water diverted through it at which time the right half of the river was coffer-damned, and then the spillway structure was built. The lower portion of the powerhouse was used as the diversion structure. Once the concrete powerhouse and spillway structure were completed construction of the embankment dam was undertaken. Concrete work was completed in the summer and fall of 1924.

Prior to 2019, gate operations were handled by a human operator, due to the means of opening and closing a gate. The gate used a chain hoist on a trolley system, where the operator was required to set the chain on the hoist drum and be in attendance to push the start and stop buttons on the gate hoist.

According to historic documentation on file with the FLTF, modifications to the dam began in 1924 with changes to the bridge above the spillway. A fire in 1927 damaged the embankments, shown in Photo 5 below, however beyond historical photographs there is no record of the details of the impacts of the fire on the Smallwood Dam. In 1985 the spillway gates were repaired, and in 1998, the left embankment was cleared of vegetation, regraded, and flattened. In 1998 the spillway was upgraded. In 2001, the left embankment was modified, and sheet piling was added in 2013. In 2014, approximately 260 feet of the overflow section just left of the SSP training wall was raised to crest elevation 711.5 feet and a 20-foot-wide rip-rap channel was also constructed along the SSP training wall to prevent scour along the base of the wall during an extreme flood event that could overtop the embankment left of the SSP. There is no record of improvements or repairs made to the right embankment, other than installation of the SSP cutoff. In 2016 the plant facility was modified. Post-construction, modifications were performed on the Smallwood Dam embankment and Routine structural repairs included continual concrete repair on the structures, as well as gate repairs. All pre-disaster modifications were performed by Wolverine Power Company, followed by Boyce Hydro.

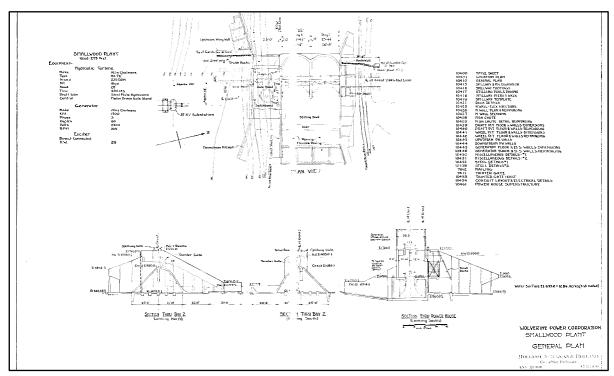


Figure 3. Original Plan Drawings of the Smallwood Dam, March 1924.



Photo 4. Smallwood Dam During Construction, 1924.



Photo 5. Smallwood Dam Post-Fire, 1927

There are 35 dams in Gladwin County, Michigan. There are more than 50 hydroelectric dams in Michigan, which provide approximately 1.5% of Michigan's energy. Four of these facilities are listed on the NRHP. Cooke Dam, constructed on the Au Sable River in 1911, is listed due to presence of associated outbuildings, including a powerhouse, substation, storage shed, and Classic-Revival style Attendant's House. The Croton Dam on the Muskegon River is listed on the NRHP as it attracted international attention in 1907 due to the 110,000-volt transmission line, which was the highest in use at that time. The Five Channels Dam, constructed in 1912, is listed on the NRHP as a District, which was known for its associated 45-acre worker's camp, which is also listed on the NRHP as an archaeological site. The Hardy Dam was constructed in 1931 and at the time was one of the largest earthen dams in the United States. Two additional dams are listed as Michigan State Historic Sites, including the Rugg Pond Dam constructed in 1904 on the Rapid River which was constructed with materials delivered via wagon and lists Ernest Hemingway as a notable guest. The other, the Mio Dam on the Au Sable River, was constructed in 1916 and is unique for its conduit spillways underneath the powerhouse foundation which paved the way for design innovations in icy rivers.

Of the hydroelectric dams in Michigan, most were constructed in the second quarter of the 20th century. To determine the presence of distinguishing features that might inform the significance of the Smallwood Dam, Merjent reviewed four other dams designed by Holland, Ackerman and Holland, all constructed between 1922 and 1927. These dams include the Chalk Hill Dam, the Victoria Dam, the Union City Dam, and the Quaker Mill Dam (see Table 2 below).

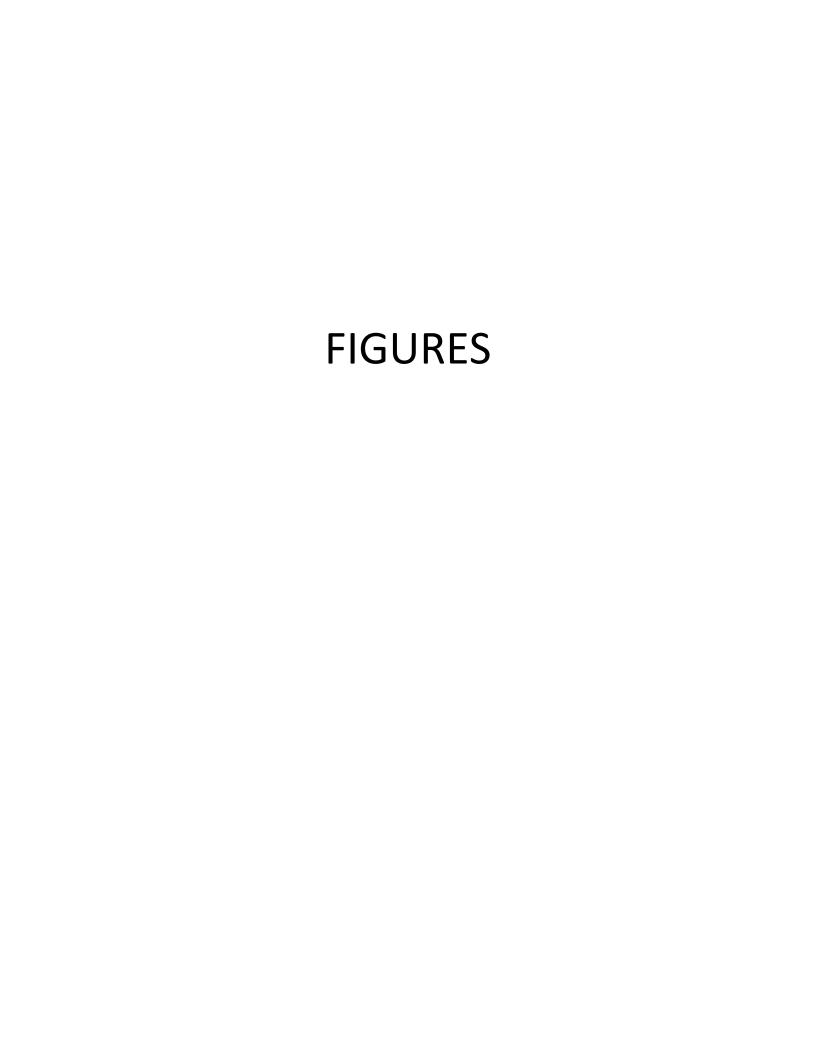
TABLE 2. List of notable Holland, Ackerman and Holland Dams						
Union City Dam	Michigan	Benjamin Douglas Company	"municipality owned"	Tainter Gates (5)		
Quaker Mill Dam	Iowa	Unknown	Iowa Electric Company	One Tainter Gate		
Smallwood Dam	Michigan	Holland, Ackerman, and Holland	Wolverine Power Company	Tainter Gates (2)		
Chalk Hill Dam	Wisconsin	Siems, Helmer, Shaffner, Inc.	Northern Electric Company	Tainter Gates (11)		
Victoria Dam	Michigan	Price Brothers Company	Copper Range Company	No Tainter gates		

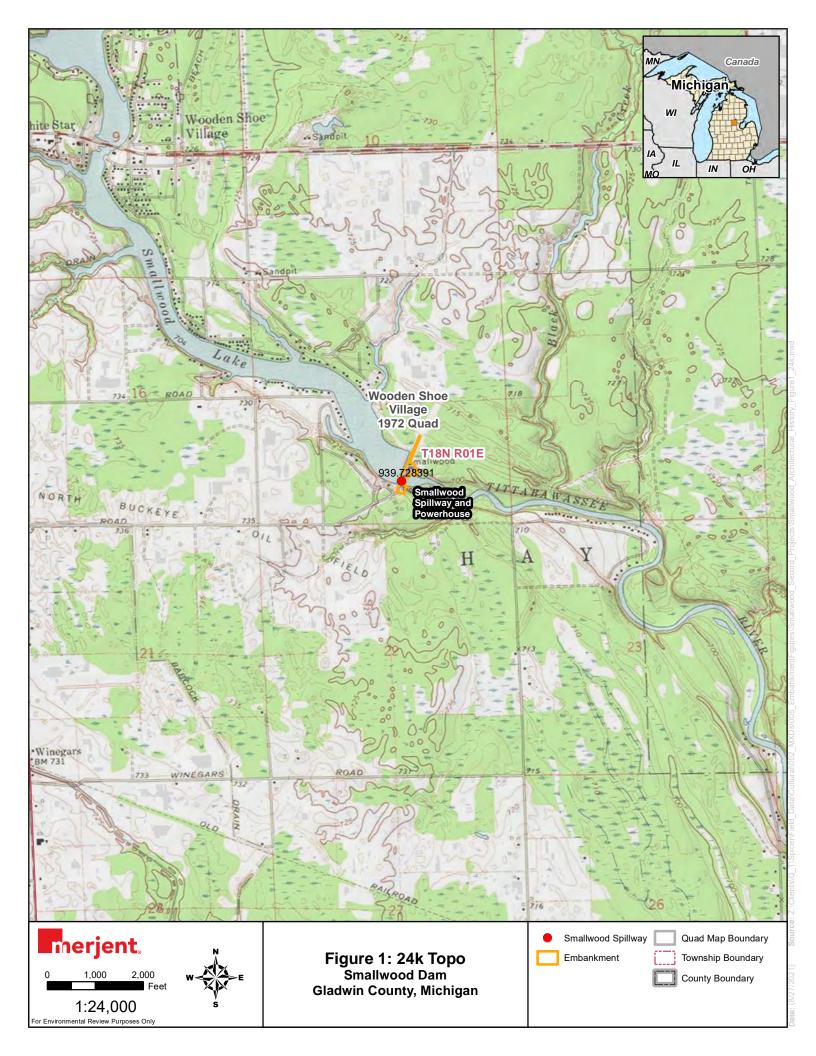
Other properties designed by Holland, Ackerman and Holland, exhibit unique architectural elements such as stained glass within the powerhouse (Chalk Hill Dam) or sweeping arches (Victoria Dam). The Quaker Mill Dam, demolished in 2017, was previously listed on the NRHP due to a few unique characteristics, namely a monumental concrete fishway and a single Tainter gate with a manually operated hoist. While both the Chalk Hill Dam and the Tobacco River Spillway are hollow, the hollow nature of the Chalk Hill Dam was utilized to access the powerhouse, which is not present at the Smallwood Dam. The Chalk Hill Dam was also distinctive due to its large concrete fishway, its size (early 300 feet wide), and the decorative elements within the powerhouse (see Photo 6). Other than some being hollow and/or having multiple Tainter gates, unique architectural features and associated structures set these facilities apart from the Smallwood Dam.

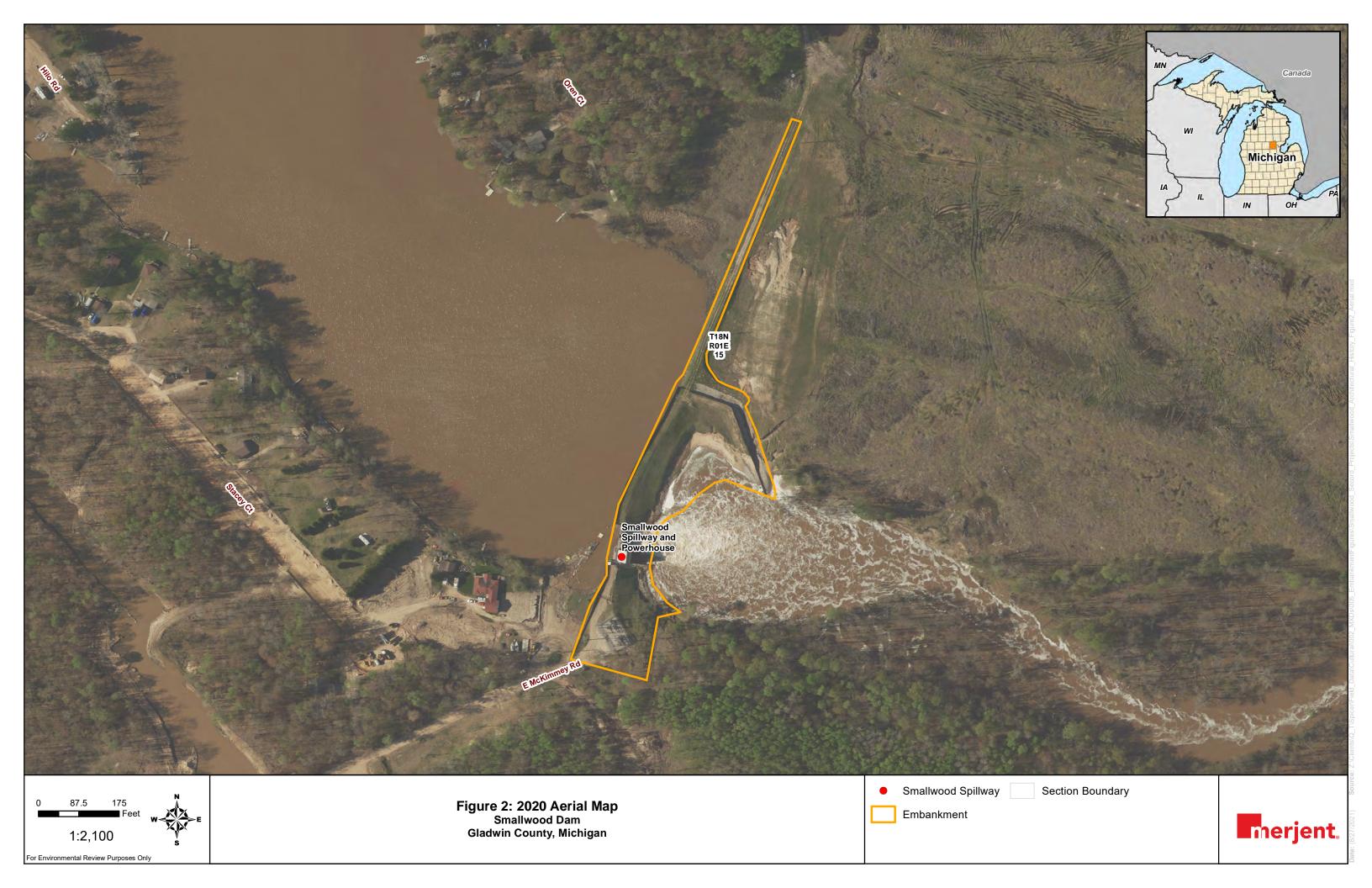


Photo 6. Chalk Hill spillway (left) and powerhouse (right).

When considering the eligibility of the Smallwood Dam, the National Register Criteria for Evaluation were considered (36 C.F.R. § 60.4). It is of our opinion that the Smallwood Dam is not eligible for inclusion on the NRHP, either in and of itself or as a contributing property of a historic district. While the powerhouse and spillway remain intact, continual repairs have modified the original structures. Regardless of impacts to the structural characteristics, the Smallwood Dam lacks unique architectural elements present among other dam spillways, powerhouses, or embankments (particularly those listed on the NRHP with associated powerhouses or other notable engineering elements), it is our professional opinion that the Smallwood Dam does not meet the criteria established by the Secretary of the Interior as detailed in 36 C.F.R. § 60.4.



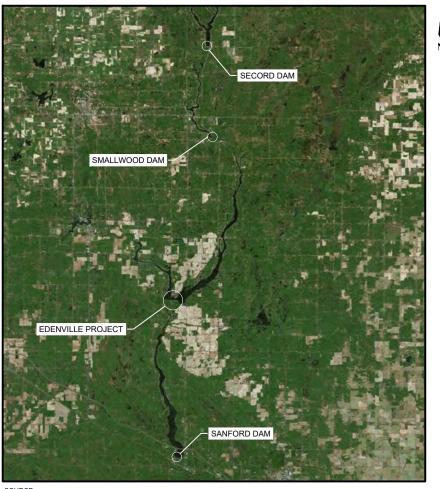




APPENDIX A

SMALLWOOD DAM CONCEPTUAL DESIGN





SOURCE: AERIAL IMAGE TAKEN FROM GOOGLE EARTH

SITE LOCATION

GLADWIN COUNTY, MICHIGAN FOUR LAKES TASK FORCE FERC PROJECT NO. 10810



SITE AERIAL (NOT TO SCALE)

PREPARED FOR:

FOUR LAKES TASK FORCE 233 E. LARKIN MIDLAND, MI 48640

PREPARED BY:

SHEET INDEX

12

13

15

16

17

SHEET NO. DRAWING NO. TITLE G-01

G-02

C-01

C-02

C-03

S

C-05

C-06

C-07

C-08

C-09

C-10

C-11

C-12

C-13

C-15

COVER SHEET AND SITE LOCATION

SITE PLAN - EXISTING CONDITIONS

OUTLET WORKS - DEMOLITION PLAN

OUTLET WORKS - DEMOLITION SECTION

SITE PLAN - PROPOSED MODIFICATIONS

OUTLET WORKS - MODIFICATION PLAN VIEW

PRIMARY SPILLWAY - CREST GATE DETAILS

POWERHOUSE - MODIFICATIONS SECTION

AUXILIARY SPILLWAY - PROPOSED PLAN VIEW

PRIMARY SPILLWAY - MODIFICATIONS SECTION

NORTH TRAINING WALL - MODIFICATIONS SECTION

NORTH TRAINING WALL - MODIFICATIONS DETAILS

EMBANKMENTS - MODIFICATIONS SECTIONS (SHEET 1 OF 2)

EMBANKMENTS - MODIFICATIONS SECTIONS (SHEET 2 OF 2)

AUXILIARY SPILLWAY - PROPOSED SPILLWAY & CHANNEL CROSS SECTIONS,

DWG. NO.

G-01 SHEET NO.

OUTLET WORKS - EXISTING CONDITIONS PLAN

OUTLET WORKS - TEMPORARY COFFERDAMS PLAN

GENERAL NOTES AND LEGEND

GEI CONSULTANTS OF MICHIGAN. P.C. 10501 WEST RESEARCH DRIVE G100 MILWAUKEE, WI 53226 (414) 930-7534



SPICER GROUP INC. 230 S. WASHINGTON AVE. SAGINAW, MI 48607 TEL. (989) 754-4717 FAX. (989) 754-4440



				DRAFT
				DRAFI
0	XX/XX/XX	CONCEPTUAL DESIGN SUBMITTAL	XXX	
NO.	DATE	ISSUE/REVISION	APP	

GEI PROJECT NO. 2002879

THIS DOCUMENT, AND THE IDEAS AND DESIGNS INCORPORATED HEREIN, IS AN INSTRUMENT OF PROFESSIONAL SERVICE, IS THE PROPERTY OF GEI CONSULTANTS AND IS NOT TO BE USED, IN WHOLE OR IN PART, FOR ANY OTHER PROJECT WITHOUT THE WRITTEN AUTHORIZATION OF GEI CONSULTANTS.

GENERAL

SPACIAL DATUM INFORMATION

- VERTICAL: NATIONAL GEODETIC VERTICAL DATUM OF 1929 (NGVD29). HORIZONTAL: NORTH AMERICAN DATUM OF 1983 (NAD83), MICHIGAN STATE PLANE,
- CENTRAL ZONE:

 A CONVERSION OF +5.8' IS REQUIRED WHEN CONVERTING VERTICAL DAM DATUM TO NGVD29 (E.G., HEADWATER ELEVATION AT DAM DATUM IS 699.0' AND AT NGVD29 DATUM IS 704.8').
- A CONVERSION OF -0.541' IS REQUIRED WHEN CONVERTING VERTICAL NGVD29 DATUM TO NAVD88 DATUM.

 CONTROL MONUMENTS ON-SITE SHALL BE REFERRED TO CONFIRM HORIZONTAL
- AND VERTICAL MEASUREMENTS.

BASEMAP DATA

- SITE TOPOGRAPHY AND AERIAL IMAGE OBTAINED DRONE FLIGHT PERFORMED BY SPICER
- COVER SHEET AERIAL IMAGES OBTAINED FROM GOOGLE EARTH REPRESENT CONDITIONS IN JUNE, 2018.
- OBTAINED FROM BOYCE HYDRO:
- ORIGINAL CONSTRUCTION DRAWINGS
- EXHIBIT F LICENSE DRAWINGS

DESIGN PARAMETERS

- NORMAL RESERVOIR ELEVATION 704.8' (+0.3' / -0.4')
- WINTER RESERVOIR OPERATIONS: MINIMUM 701.8'
- ORDINARY HIGH WATER MARK ELEVATION 675.2'

DESIGN REFERENCE STANDARDS

- (USBR, 1987) UNITED STATES DEPARTMENT OF THE INTERIORER, BUREAU OF RECLAMATION, "DESIGN OF SMALL DAMS", 1987.
- (USACE, 1995) UNITED STATES ARMY CORPS OF ENGINEERS, ENGINEERING AND DESIGN,
 "CONSTRUCTION CONTROL FOR EARTH AND ROCK-FILL DAMS", EM 1110-2-1911, 1995.
- (ACI, 2001) AMERICAN CONCRETE INSTITUTE, "CONTROL OF CRACKING IN CONCRETE
- STRUCTURES" (ACI 224), 2001.
- (USACE, 2004) UNITED STATES ARMY CORPS OF ENGINEERS, ENGINEERING AND DESIGN,
 "GENERAL DESIGN AND CONSTRUCTION CONSIDERATIONS FOR EARTH AND ROCK-FILL DAMS", EM 1110-2-2300, 2004.
- (ACI, 2006) AMERICAN CONCRETE INSTITUTE, "CODE REQUIREMENTS FOR ENVIRONMENTAL ENGINEERING CONCRETE STRUCTURES" (ACI 350), 2006
- (ACI, 2011) AMERICAN CONCRETE INSTITUTE, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318), 2011.
- (FERC, 2016) FEDERAL ENERGY REGULATORY COMMISSION, ENGINEERING GUIDELINES FOR EVALUATION OF HYDROPOWER PROJECTS (MOST RECENT VERSIONS)

ABBREVIATIONS

BO = BOTTOM OF

C = GENTER LINE

MM = MOVEMENT MONUMENT

CLSM = CONTROLLED LOW-STRENGTH MATERIAL

CONC = CONCRETE

CONT = CONTINUOUS

CTRD = CENTERED D/S = DOWNSTREAM

EO = EDGE OF

EX = EXISTING

EF = EACH FACE

EL = ELEVATION (FEET)

HW = HEADWATER

MAX = MAXIMUM

OC = ON CENTER OCEW = ON CENTER EACH WAY

OHWM = ORDINARY HIGH WATER MARK

PL = PLATE

PMF = PROBABLE MAXIMUM FLOOD

SDF = SPILLWAY DESIGN FLOOD

SSP = STEEL SHEET PILE

STD = STANDARD

STIFF = STIFFENER

TBD = TO BE DETERMINED

TO = TOP OF

TW = TAILWATER

TYP = TYPICAL

UON = UNLESS OTHERWISE NOTED

U/S = UPSTREAM

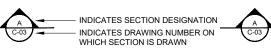
VIF = VERIFY IN FIFI D

WL = WETLAND

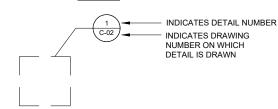
W/ = WITH

SECTION AND DETAIL LEGEND

SECTION



DETAIL



LINETYPE LEGEND

	CENTERLINE
	WATER ELEVATION
O/E	OVERHEAD ELECTRIC LINES
xx	FENCE LINE (STEEL)
	FENCE LINE (WOOD)
CATV	UNDERGROUND CABLE
GAS	GAS LINE
	EDGE OF ROADWAY (UNPAVED)
———	ROADWAY CENTERLINE
	BURIED PIPING
	SILT FENCE
750	EXISTING MAJOR CONTOURS
	EXISTING MINOR CONTOURS
	DESIGN MAJOR CONTOURS
	DESIGN MINOR CONTOURS
	SHEETPILE

SYMBOLS LEGEND

WATER ELEVATION

→ FLOW DIRECTION

H:1V CUT SLOPE

1H:1V FILL SLOPE

POWER POLE α

SOIL BORING COMPLETED BY PROFESSIONAL SERVICE INDUSTRIES, 1997

BM #200 SURVEY REFERENCE MONUMENT

HATCH LEGEND

CONCRETE

DEMOLITION



PROPOSED

REINFORCED

CONCRETE

WOOD

STRUCTURE

FILTER STONE







STEEL









REINFORCED

CONCRETE CAR



EXISTING FOUNDATION

DRAINAGE

STONE

GEI Project 2002879



AGGREGATE

BERM FILL

AREA









1/1 /

CELLULAR

Attention 0 xx/xx/xxxx CONCEPTUAL DESIGN SUBMITTAL 1" then drawing is not original scale. NO. DATE APP ISSUE/REVISION

DRAFT





	Designed:	P. DREW		
	Checked:	P. DREW		
Sultants CHIGAN, P.C. H DRIVE	Drawn:	A. SAMPSON		
3226	Approved By:	B. WALTON		

P. DREW P. DREW A. SAMPSON

Four Lakes Task Force FERC Project No. 10810

Smallwood Dam Gladwin County, Michigan

GENERAL NOTES AND LEGEND

2

DWG. NO.

G-02

SHEET NO.

